

Influence of Metakaolin in High Strength Concrete of M70 Grade for Various Temperatures and Acidic Medium

D.Viswanadha Varma¹, G.V. Rama Rao²

1. Research Scholar, Andhra University College of Engineering, Visakhapatnam, A.P., India.

2. Professor, Dept of Civil Engineering, Andhra University College of Engineering, Visakhapatnam, A.P., India.

Abstract: Concrete made of cement is the most adaptive material for construction purpose. Design and Preparation of such a concrete mix with good strength and durability has been always the need of the day. Use of mineral admixture in concrete mix has made a remarkable achievement in development and design of high strength concrete.

Among many mineral admixtures available, Metakaolin (MK) is a mineral admixture, whose potential is not yet fully tested and only limited studies have been carried out in India on the use of MK for the development of high strength concrete. MK is a supplementary cementations material derived from heat treatment of natural deposits of kaolin. MK shows high pozzolanic reactivity due to their amorphous structure and high surface area.

In this present study Metakaolin is replaced with 0, 10, 15, 20, 25 and 30 % of cement to observe the compressive strength of M70 grade concrete. Conclusions are made from the various results and the discussions there on to identify the effect of partial replacement of cement by MK in the design concrete mix. The results conclude that, the use of Metakaolin Concrete (MKC) has improved the performance of concrete under various conditions

Key words: Metakaolin, High Strength Concrete, Temperatures and Acidic Medium.

I. Introduction

Concrete is one of the most widely used man-made construction material in the world. Metakaolin is the cementations material used as an admixture to produce high strength concrete. Optimal quality of Metakaolin for M70 grade of concrete has been worked out, which can replace the cement in order to get better strength and durability. Also identification of the drying shrinkage and permeability characteristics of blended cement has been done. Jipingbai studied that when metakaolin is used as a partial replacement for Portland cement, tends to improve both the mechanical properties and durability of concrete. Friars and Cabrera investigated the relation between the pore size distribution and degree of hydration of metakaolin based cement pastes.

Metakaolin is the white powder of $Al_2O_3 \cdot 2SiO_2$ by dehydrating kaolin ($Al_2O_3 \cdot 2SiO_3 \cdot 2H_2O$) at an appropriate temperature (700-900°C). Kaolin is in a layered silicate structure, with the layers binding with each other via the Van Der Waal's bond, among which OH^- is bound firmly. Kaolin, when being heated in air, may experience several structural changes, and when being heated to around 600°C, the layered structure of kaolin is damaged due to dehydration to form a transient phase with a poor crystallinity, i.e. metakaolin. As the molecular arrangement of metakaolin is irregular in a thermodynamic metastable condition, it is cementations under an adequate excitation. With a high activity, metakaolin can be used to manufacture cementations materials and mix high-strength high-performance concrete.

II. Methodology

Based on the preliminary investigations carried out, the experimental investigation is planned as under. To obtain the mix proportions of OPC concrete for M70 by Entropy and Shack lock's Empirical graphs. To calculate the mix proportions with partial replacements such as 0%, 10%, 15%, 20%, 25% and 30% of Metakaolin with concrete. Preparations of testing specimens are as follows.

1. To prepare concrete specimens such as cubes (150 x 150 x 150) for durability studies in laboratory with 0%, 10%, 15%, 20%, 25% and 30% replacement of OPC with Metakaolin for M70 grade concrete.
 2. To prepare the concrete specimens such as cubes (150 x 150 x 150mm) for compressive strength, cylinders (150 x 300mm) for split tensile test, prisms (100 x 100 x 500mm) for flexural strength, cylinders (150 x 300mm) for stress-strain curve with 0% and 15% replacement of OPC with Metakaolin for M70 grade concrete for temperature study i.e., for 100°C, 200°C, 300°C, 400°C and 500°C. To cure the specimens for 28 days.
 3. To evaluate the mechanical characteristics of concrete such as compressive strength, split tensile, flexural strength, stress-strain curve.
-

4. To evaluate the durability studies of M70 grade Metakaolin replacement concrete with 0.5% and 1% concentrations of HCl and H₂SO₄.
5. To evaluate the temperature studies of M70 grade MKC at an exposure of 100°C, 200°C, 300°C, 400°C and 500°C for 1hr, 2hr and 3hr duration. To evaluate and compare the results.

III. Results And Discussion

The tests were carried out to obtain compressive strength, split tensile strength, flexural strength and stress-strain curve of M70 grade concrete. The specimens are tested for 28 days for 0%, 10%, 15%, 20%, 25% and 30% replacement of MK for compressive strength and the specimens are tested for 28 days for 0% and 15% replacement of MK for flexural strength, stress-strain curve, split tensile strength. These are presented in tables and graphs were plotted correspondingly.

In the present experimental work the specimens exposed to temperature undergo physical changes and weight loss. The free moisture content is lost initially, followed by physical adsorption of water.

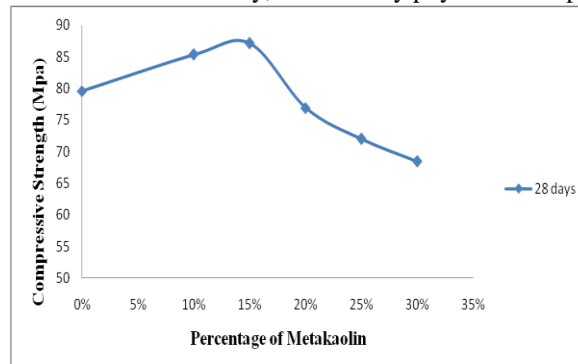


Figure 1. Compression strength of concrete vs. % of Metakaolin

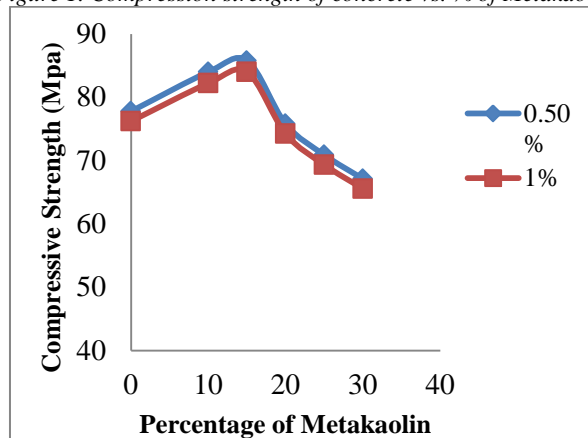


Figure 2. Compressive Strength of concrete vs. % of MK at 0.5% and 1% HCl

From the above Fig: 2, it is observed that at 15% replacement of cement with MK, concrete attains maximum compressive strength when exposed to 0.5%, 1% HCl at the age of 28 days. When the replacement exceeds 20%, the compressive strength is found to be decreasing slightly.

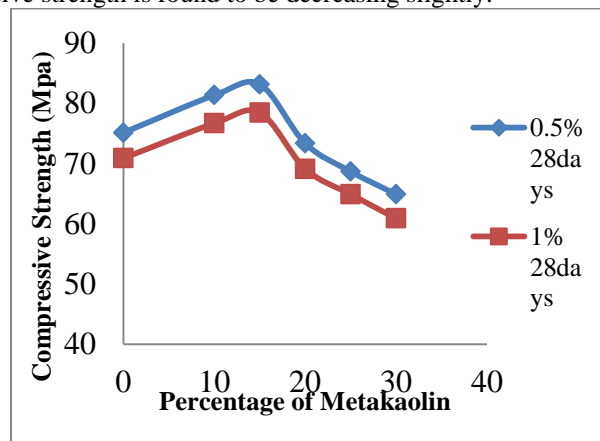


Figure 3. Compressive Strength of concrete vs. % of Metakaolin at 0.5%, 1% H₂SO₄

From the above graph 3, it is observed that at 15% replacement of cement with MK, concrete attains maximum compressive strength when exposed to 0.5%, 1% H₂SO₄ at the age of 28 days. When the replacement exceeds 20%, the compressive strength is found to be decreasing slightly.

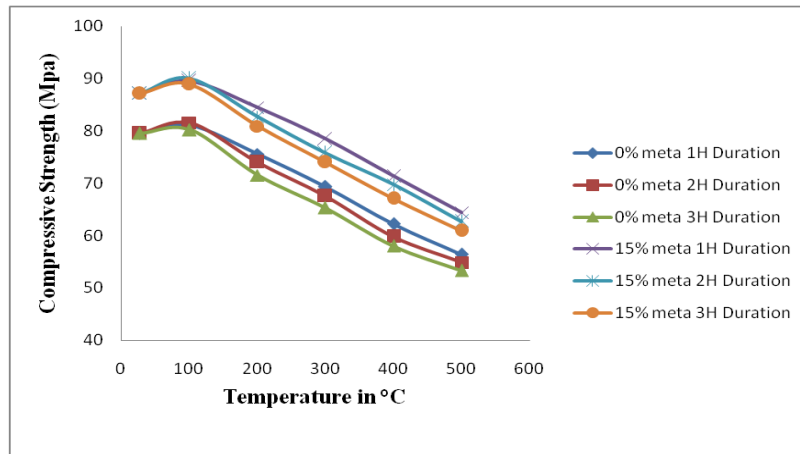


Figure 4. 28 Days Compressive Strength of concrete (%) vs. Exposed temperature (°C) of 0% and 15% MKC

From the above graph 4, it is observed that the compressive strength increases at 100°C temperature when compared to the strength obtained at normal room temperature for 0% and 15% replacement of MK. The increase in compressive strength associated with the increase in temperature is attributed to the increase in the surface forces between gel particles (Vander wall forces) due to the removal of moisture content. The compressive strength decreases from 100°C to 200°C and further decreases at 500°C.

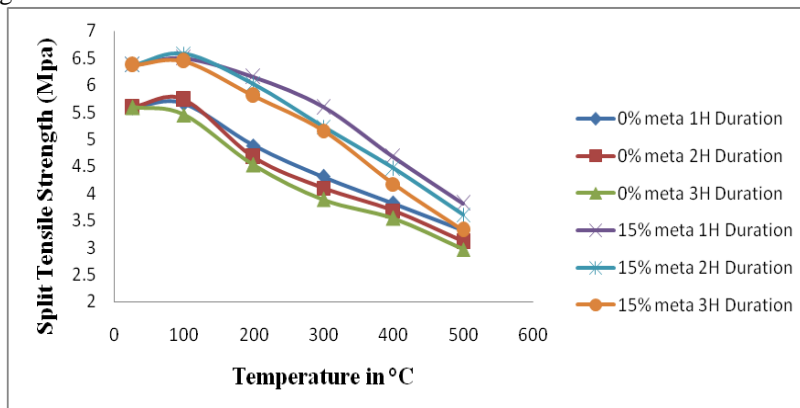


Figure 5. 28 Days Split Tensile Strength of concrete (%) vs. Exposed Temperature (°C) of 0% and 15% MKC

From the above graph 5, it is observed that the split tensile strength increases at 100°C temperature when compared to the strength obtained at normal room temperature for 0% and 15% replacement of MK. The increase in split tensile strength associated with the increase in temperature is attributed to the increase in the surface forces between gel particles (Vander wall forces) due to the removal of moisture content.

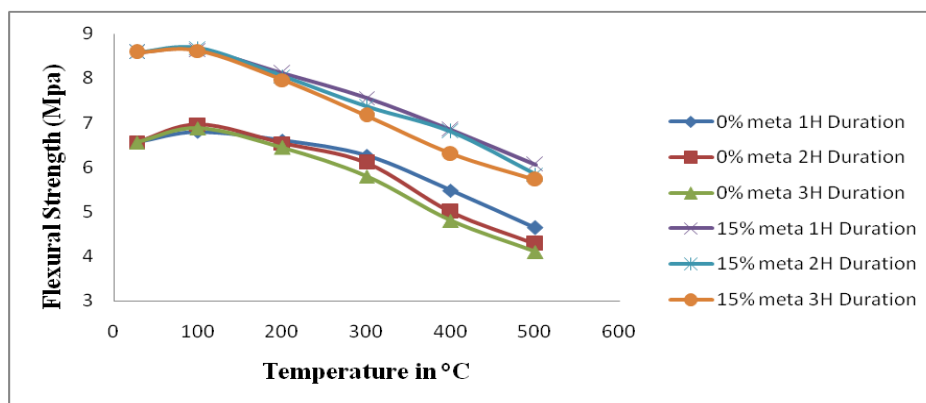


Figure 6. 28 Days Flexural Strength of concrete (%) vs. Exposed Temperature (°C) of 0% and 15% MKC

From the above graph 6, it is observed that the flexural strength increases at 100°C temperature when compared to the strength obtained at normal room temperature for 0% and 15% replacement of MK. The increase in flexural strength associated with the increase in temperature is attributed to the increase in the surface forces between gel particles (Vander wall forces) due to the removal of moisture content.

The stress-strain behavior of cylinder specimens for 0% and 15% of metakaolin cured for 28 days age and subjected to elevated temperature from 100 to 500°C apart from room temperature were as shown below. The various graphs plotted are as shown in Fig. 7 to 12.

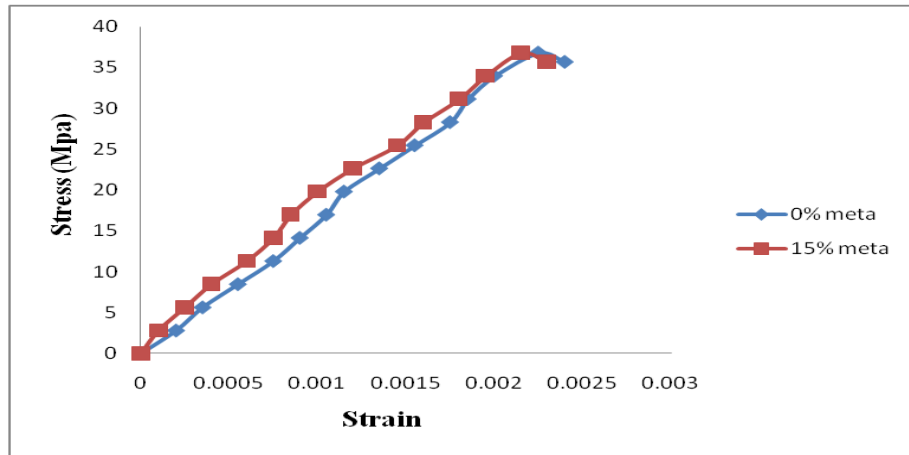


Figure 7. Stress-strain curve of concrete at room temperature

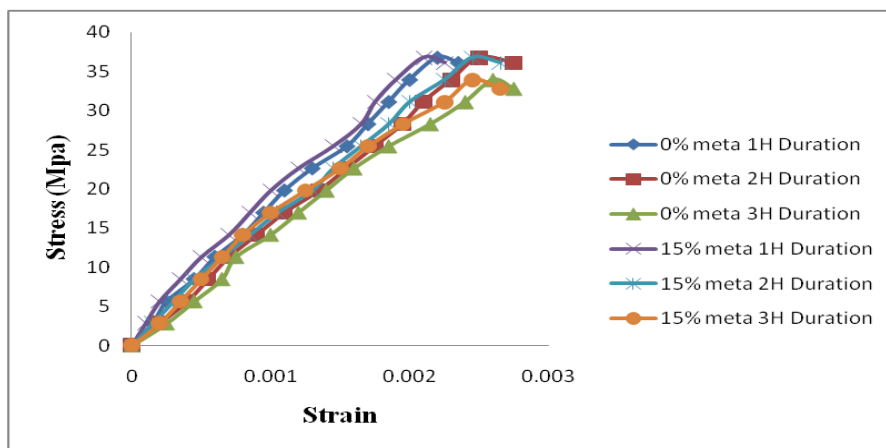


Figure 8. Stress-strain curve of concrete exposed to 100°C for different exposure durations

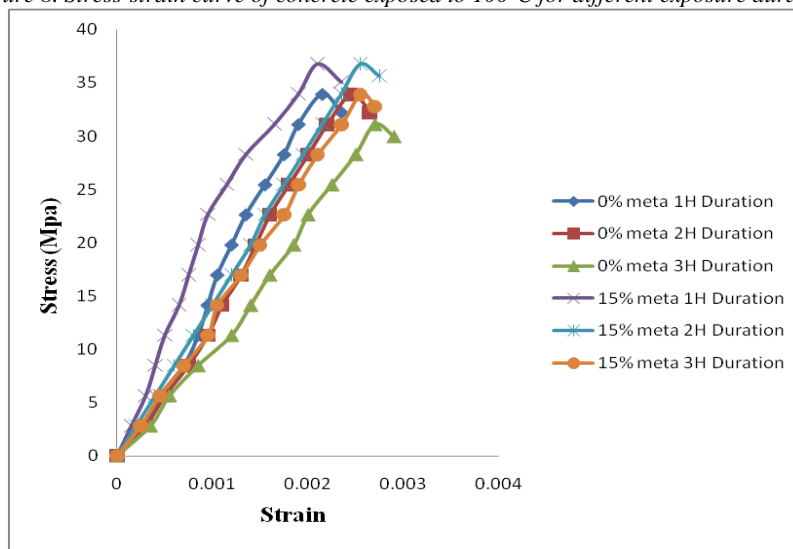


Figure 9. Stress-strain curve of concrete exposed to 200°C for different exposure durations

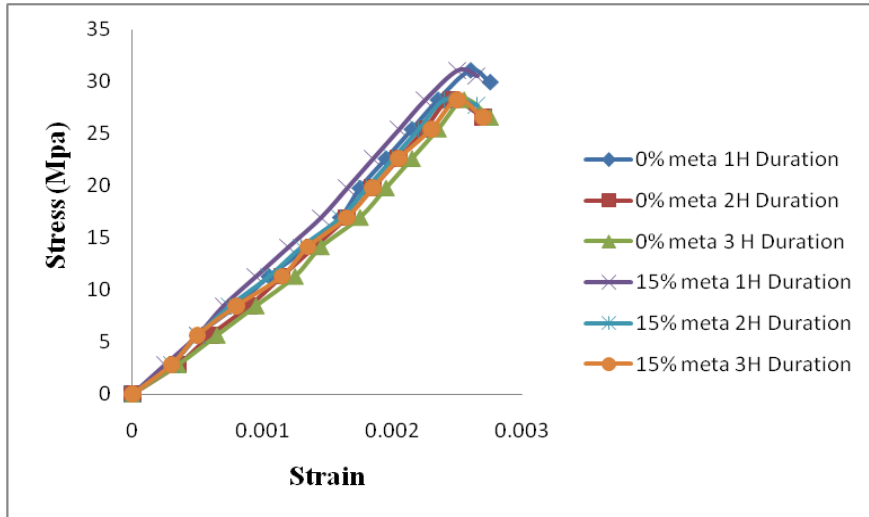


Figure 10. Stress-strain curve of concrete exposed to 300°C for different exposure durations

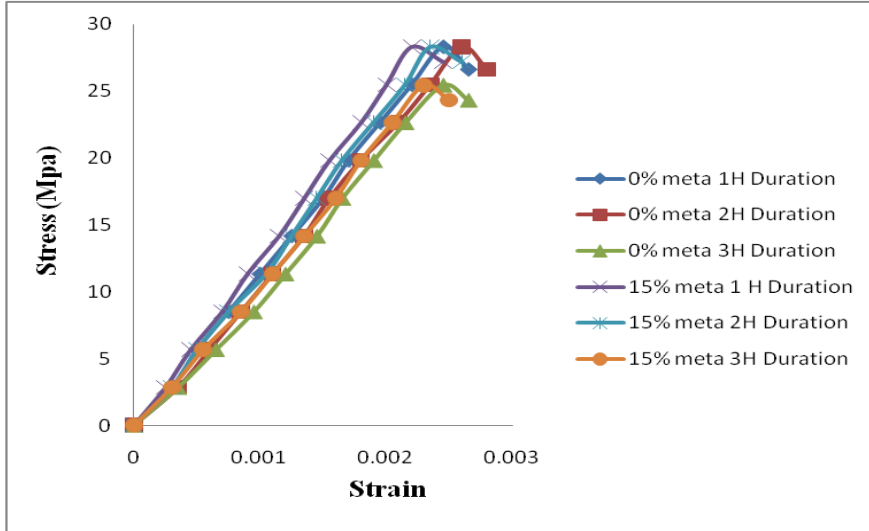


Figure 11. Stress-strain curve of concrete exposed to 400°C for different exposure durations

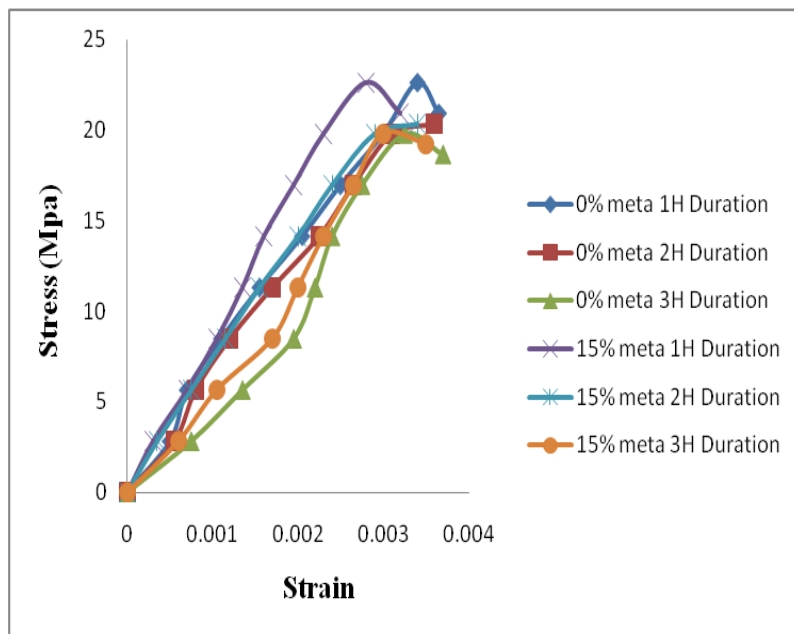


Figure 12. Stress-strain curve of concrete exposed to 500°C for different exposure durations

IV. Conclusions

Based on the experimental investigation carried out, the following conclusions are made.

1. Workability of concrete decreases with the increase in Metakaolin replacement level.
2. The compressive strength, flexure strength and split tensile strength of conventional concrete and concrete with MK as partial replacements are compared and observed and concluded that the strength of the conventional concrete is slightly lower than the MKC.
3. The compressive strength of concrete is increased when cement is replaced with Metakaolin. The compressive strength is maximum at 15% of replacement.
4. The split tensile strength of concrete is increased when cement is replaced with Metakaolin. The split tensile strength is maximum at 15% of replacement.
5. The flexure strength of concrete is increased when cement is replaced with Metakaolin. The flexure strength is maximum at 15% of replacement.
6. At room temperature and 100°C exposure, the stress-strain relationship is similar to the conventional concrete & MKC behavior. However the trend is different for temperature exposure of 200°C to 500°C.
7. The compressive strength of concrete showed better result at 15% replacement of MK for 0.5% and 1% HCl at the age of 28 days of strength.
8. The compressive strength of concrete showed better result at 15% replacement of MK for 0.5% and 1% H₂SO₄ at the age of 28 days of strength.
9. The effect of HCl on strength of the Metakaolin concrete is lower than the effect of H₂SO₄.
10. The strength increases at 100°C temperature and thereafter it starts losing its strength as the temperature increases.

References

- [1]. **Abdul Razak** (2005) "Strength estimation model for high-strength concrete incorporating metakaolin and silica fume", Cement & Concrete Research 35, pp 688-695.
- [2]. **Bamonte p** (2010) "Thermal and Mechanical Properties at High Temperature of a Very High-Strength Durable Concrete" Journal of Materials in Civil Engineering ASCE, pp 545-555.
- [3]. **Bo Wu, Xiao-Ping Su, Hui Li and Jie Yuan** (2002) "Effect of High Temperature on Residual mechanical properties of Confined and Unconfined high strength concrete", ACI-Materials journals, Vol.99, No.4, pp 231-239.
- [4]. **Beulah M** (2012) "Effect of Replacement of Cement by Metakal on the Properties of High Performance Concrete Subjected to Hydrochloric Acid Attack", IJERA, Vol.2, Issue 6, pp.033-038.
- [5]. **Chi-Sun Poon** (2003) "Performance of metakaolin concrete at elevated temperatures", Cement & Concrete Research 25, pp 83-89.
- [6]. **Chi-Sun Poon** (2006) "Compressive strength, chloride diffusivity and pore structure of high performance metakaolin and silica fume concrete", Cement & Concrete Research 20 pp 858-865.
- [7]. **Dinakar P** (2011) "High reactive metakaolin for high strength and high performance concrete" The Indian Concrete Journal, pp 28-34.
- [8]. **Frias M** (2000) "Pore size distribution and degree of hydration of Metakaolin-cement pastes", Cement & Concrete Research Vol.30, pp 561-569.
- [9]. **IS 516: 1959**, "Method of test for strength of concrete", Bureau of Indian Standards, New Delhi.
- [10]. **IS 12269: 1987**, "Specification for 43 Grade Ordinary Portland Cement", Bureau of Indian Standards, New Delhi.
- [11]. **Khatib and S.Wild**(1996) "Pore size distribution of Metakaolin paste" in cement and concrete research, ICJ Vol. 26 No. 10, pp 1545-1553.
- [12]. **Khatib and Wild**(1998). "Sulphate Resistance of Metakaolin Mortar", Cement and Concrete Research, ICJ Vol .28. No. 1, pp 83-92.
- [13]. **Long T. Phan** (2000) "Fire Performance of High Strength Concrete" Journal of Materials in Civil Engineering ASCE, May 8-10.
- [14]. **Nabil M. Al-Akhras** (2006) "Durability of metakaolin concrete to sulfate attack", Cement and Concrete Research 36, pp- 1727-1734.
- [15]. **Rama Rao G.V, Seshagiri Rao M.V.** (1997), "Improvement in Durability Characteristics of concrete using Pozzolanic Material as Admixture", Proceedings of International Conference on maintenance & Durability of Concrete Structures, Mar 4-6.
- [16]. **Rafat Siddique** (2010) "Effect of metakaolin on the near surface characteristic of concrete", Materials and Structures 44, pp 77-88.
- [17]. **Shetty, M.S.** "Concrete Technology", S.Chand & company ltd, New Delhi.
- [18]. **Srinivasa Rao K, PothaRaju.M&Raju.P.S.N** (2004) "Effect of Age on HSC on Residual Compressive Strength under Elevated temperatures", International Conference on Advances in Concrete and Construction, pp 733-741.
- [19]. **Vikas Srivastava** (2012) "Effect of Silica Fume and Metakaolin combination on concrete", IJCSE, Vol.2, Issue 3, PP 893-898.
- [20]. **Xia oquian and Zongjinli**(2001). "The relationships between stress and strain for high performance concrete with Metakaolin", Cement & concrete research 31, pp1607-1611.