

## Effect of Fly ash on Strength Characteristics of Roller Compacted Concrete Pavement

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**Abstract:** In this paper the behaviour of roller compacted concrete pavement with fly ash is examined. In the preparation of fly ash roller compacted concrete (FRCC), the maximum aggregate size was 20mm in all of the mixtures which were designed using maximum density method. The cement content of the control mixture was 318kg/m<sup>3</sup>. Cement was replaced at various percentage levels such as 20%, 40% and 60% besides control concrete was also prepared for comparison purpose. Compressive strength, split tensile strength, flexural strength were determined at 3, 7 and 28 days. Test results indicate that the mixtures where cement was substituted with fly ash, increasing the fly ash content caused reduction in compressive, splitting tensile and flexural strength values at all of the ages up to 28 days.

**Keywords:** RCCP, Mix Design, Strength, Cement content, w/c Ratio, admixtures

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### I. Introduction

Roller compacted concrete (RCC) is a dry mixture of coarse aggregates, fine aggregates, cement and water compacted by vibratory rollers (ACI Committee 325, 1995)[1]. The use of RCC for pavements is relatively a new technology and is still under development. The first RCCP was constructed in Canada in 1976 at the log-sorting yard at Cayuse on Vancouver island, by the British Columbia forest company (BCFP) where 4 acres of RCCP were constructed and then was doubled in 1978 expansion[2]. Following this success, several projects utilising RCCP were constructed in America, Spain, Australia and elsewhere[1,2,3]. RCC is a friendly pavement material in which incorporated by products from industries. Its use in RCC can be achieved by directly adding these materials to the mixture proportions, or by replacing fine mineral aggregate content.

Fly ash (FA) is the residue collected from burning smoke flue. The main useful effect of FA in concrete consists of three aspects, often called morphologic effect. The morphologic effect states that there are many micro beads in FA working as "lubricating balls" when incorporated in fresh concrete. Hence it benefits the fluidity. The micro aggregate effect of FA states that micro beads in FA can disperse well in concrete and combined firmly with gel produced in cement hydration and thus promote concrete density.[4] Various mineral admixtures such as fly ash are used in order to improve some mechanical and physical properties of concrete and to reduce its cost. Fly ash is obtained as a waste product during the combustion of pulverized coal in thermal power plants. Fly ash is used as an admixture in cement and concrete because of its pozzolanic and/or self-cementitious nature. The effect of fly ash on the mechanical properties of RCC mixtures was studied by several investigators. The influence of high amounts of fly ash used in roller compacted concrete on compressive and flexural strengths has been investigated by Cao et al[5]. In this study 45-95 wt% of cement was replaced with fly ash in six different proportions. The cement content of the control mix was 300kg/m<sup>3</sup>. It was observed that there was a decrease in strength at early ages and an increase in strength at later ages while the amount of fly ash increased from 0% to 55%. Increase in the amount of fly ash from 55% to 95% in the mixture caused strength loss at both early and later ages.

### II. Materials And Method

#### 2.1 .Materials Used In The Research

1) Cement: Ordinary Portland cement (OPC) 53 grade conforming to Indian standard IS : 12269(1987) was used for the present experimental investigation. Its specific gravity is 3.15. The cement was tested as per the procedure given in Indian standard IS 4031(1988)

2) Fine aggregate: Natural river sand conforming to zone III as per IS: 383(1987) was used. The specific gravity is 2.65

3) Coarse aggregate: Crushed granite coarse aggregate conforming to IS:383(1987) was used. Coarse aggregate of size 20mm down having the specific gravity of 2.87 was used.

4) Class F fly ash obtained from Dr.N.T.T.P.S,Ibrahim patanam was used. Its specific gravity is 2.20

## **2.2.Mix Designing Method**

The method employed for mix design is derived from the soil compaction method, and is based on the relationship between bulk density of the dry mixture and the moisture content of RCC. Mix was designed as per ACI 211-3R-19 guide lines. In this method a series of mixture for each cementitious materials content is prepared and batched using a range of water contents. Each prepared mixture is compacted with modified proctor's effort. The maximum density and optimum water content are determined from a plot of density vs. water content for the compacted specimens at each cementitious material content. RCC specimens are made at the OMC (Optimum Moisture Content).

## **III. Test Procedure**

### **3.1casting Of Beams**

The dimensions of all the specimens are identical. The length of beams was 500mm and the cross sectional dimensions were 100mm x 100mm.The beams were casted and de-moulded after one day and are allowed to cure in water.

### **3.2 Casting Of Cubes**

The dimensions of all the specimens are identical. The length , width and depth of the cubes are 150mm. The cubes were casted and de-moulded after one day and are allowed to cure in water.

### **3.3 Casting Of Cylinders**

150x300mm cylinders for split tensile strength .The cylinders were casted and de-moulded after one day and are allowed to cure in water

### **3.4 Testing Of Specimens**

The compressive strength,Split Tensile strength,Flexural strength of 150mm cube,300x150mmcylinder,100x100x500mm prism specimens respectively were obtained at 3,7,and 28 days ages in accordance with IS:516-1959standard.



**Casted beam**



**Tested Beam**

## **IV. Test Results & Discussion**

The strength test results are presented in table 2,3,and 4.It seen that the minimum strength results at all ages belong to the mixture where 60% of the cement is replaced with fly ash. The reduction in strengths is higher at 7-day age specimens.The reduction in strength of mixture is due to fact that the contribution of fly ash

to the strength of concrete is lower than that of cement even up to 28 days. However, owing to the pozzolonic reaction of fly ash the difference between the strength of control mixture and fly ash containing mix decreased by time

Table1: Mixing proportions of FRCC in kg per one m<sup>3</sup> of concrete

M	FA	C	W	RS	CA	W/cm
F <sub>0</sub>	0	318	92.1	872	1316	0.28
F <sub>20</sub>	63	254	116.3	840.6	1267.8	0.37
F <sub>40</sub>	127	190	137.2	814.0	1227.6	0.46
F <sub>60</sub>	190	127	141.4	803.1	1211.3	0.48

M: Mix ; FA: Fly ash; W: Water; RS: River Sand; CA: Coarse Aggregate; W/cm: Ratio of water to Cementitious material

Table2: Split tensile strength test results (N/mm<sup>2</sup>)

Mix	F0	F20	F40	F60
Fly ash (%)	0	20	40	60
Test age (days)	Split tensile strength (Mpa)			
3	1.56	1.55	0.77	0.14
7	2.28	1.98	1.55	0.42
28	4.02	5.30	3.18	1.69

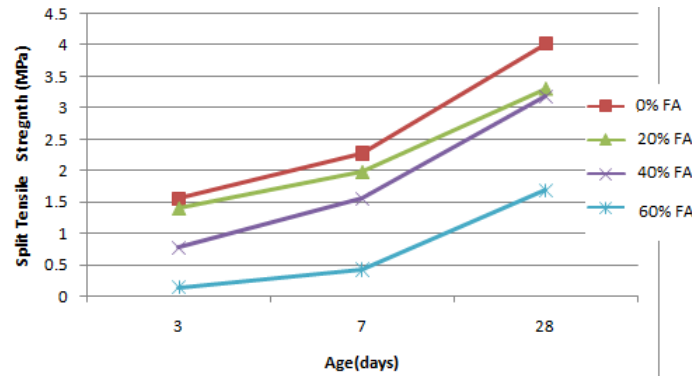


Fig 1. Split tensile strength versus age (days)

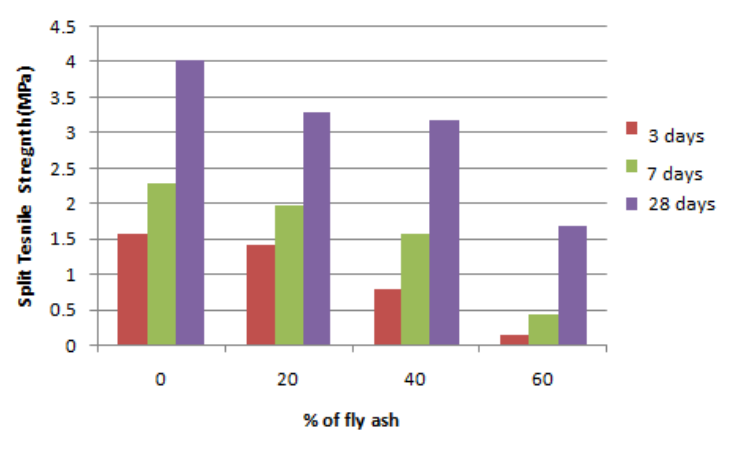


Fig 2. Split tensile strength versus fly ash percentage

Table 3: Compressive strength

Mix	F0	F20	F40	F60
Fly ash (%)	0	20	40	60
Test age (days)	Compressive strength(Mpa)			
3	18.67	16.89	11.55	3.99
7	22.22	20.00	16.44	8.88
28	33.74	29.78	24.00	16.00

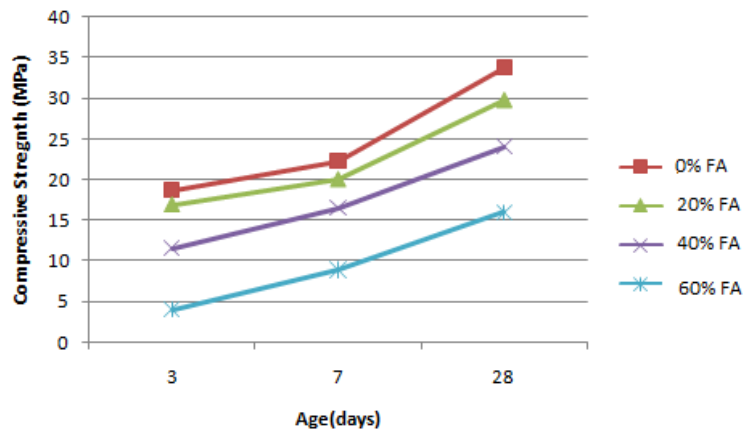


Fig 3. Compressive strength versus age

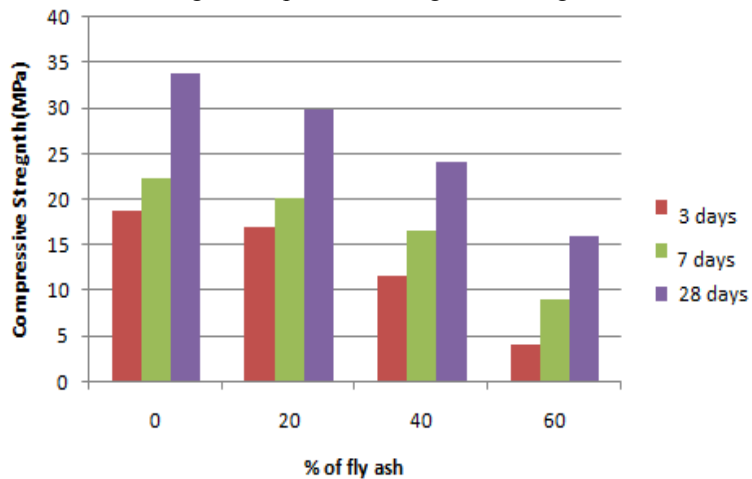


Fig4. Compressive strength versus flyash percentage

Table4: Flexural strength

Mix	F0	F20	F40	F60
Fly ash (%)	0	20	40	60
Test age (days)	Flexural strength(Mpa)			
3	3.8	3.4	3.2	0.4
7	4.4	4.4	4.0	2.2
28	6.8	7.1	5.4	4.2

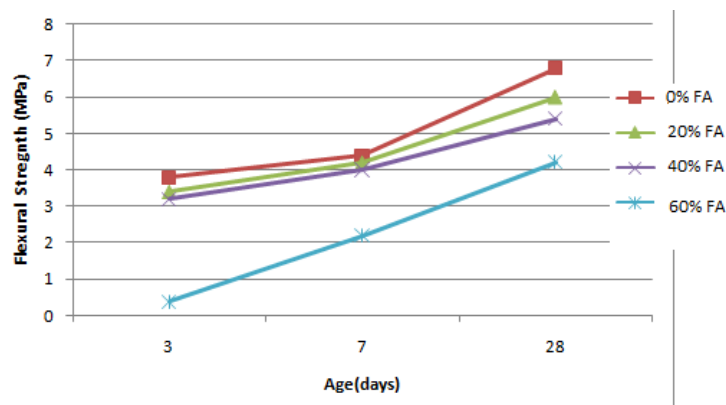


Fig 5. Flexural strength versus age

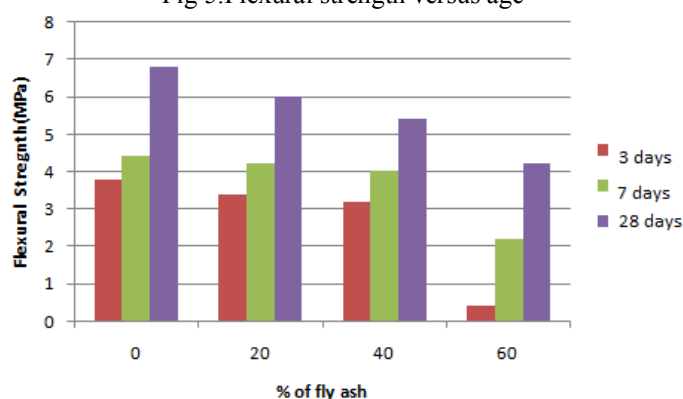


Fig 5. Flexural strength versus percentage of fly ash

## V. Conclusion

For the materials used and test methods applied the following conclusions can be drawn. When increasing the fly ash content and increase in water cement ratio to the mixture the unit weight of mix decreases.

In mixtures where cement was partially replaced with fly ash, strength values were decreased with increasing fly ash content. The effect, in part, was attributed to the increase in water/binder ratio of the mix due to fly ash addition and in part to the lower contribution of fly ash than cement to strength even up to 28 days. When replacing 60% fly ash with the cement it causes to reduction in 7-day strength and the mix gains only 50% of its 28-days strength. However, at later ages the rate of strength of mix increases and very close to each other and independent of fly ash content of the mix.

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